

GEOTECHNICAL INVESTIGATION

FOSTER BLOCK, GOULDS ROAD ROLLESTON

SUBMITTED TO: R.D. HUGHES DEVELOPMENTS LTD PO BOX 848 CHRISTCHURCH 8140

21 December 2011

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1 INTRODUCTION

Geoscience Consulting (NZ) Ltd (Geoscience) was requested by Kelvin Back of R.D. Hughes Developments Ltd to undertake a preliminary geotechnical investigation of a proposed residential subdivision site on Goulds Road, Rolleston (herein referred to as 'the site') as outlined in our proposal (ref. P11382, dated 9th November 2011).

We understand the site has recently been rezoned from rural to residential and that this report was commissioned in order to provide sufficient information for Geoscience to comment on the likely 'Technical Category'¹ into which this land will fall. This report is not intended for 224c sub-division sign off or for building consent purposes. The Selwyn District Council guidelines indicate further geotechnical testing may be required for these consents. A final report will be provided following earthworks to confirm the site technical category. We assume this will be required as a condition of subdivision consent.

Our scope of works included the following:

- Desktop study of relevant publically available geotechnical and geological publications;
- Undertake a visual inspection of the site to identify land damage following the recent earthquakes;
- Organise and technical supervision approximately 12 test pits to depths of between 1 and 4 m.
- Arrange for a sub-contractor to complete approximately 80 Mechanical Scala Penetrometer tests across the site to a target depth of 1-2 m.
- Presentation of a report outlining our findings on the prevailing ground conditions and including our opinion as to the appropriate 'Technical Category' for the site. This will include a comment on the likely liquefaction potential at the site. We will also outline what further geotechnical work we consider will be required for sub-division consent.

2 SITE DESCRIPTION

The site is located on relatively flat ground approximately 2 km southeast of the centre of Rolleston. The site is bound by Dynes Road to the northwest, Goulds Road to the West, East Maddisons Road to the southwest and farmland to the southeast and northeast (Figure 1). The site is currently used for a mixture of grazing and cropping. There are no significant watercourses in the area.

The Department of Building and Housing (DBH) has recently divided the CERA 'Green Zone' into the following Technical Categories¹:

- TC1 ('Grey') where "future land damage from liquefaction is unlikely, and ground settlements are expected to be within normally accepted tolerances";
- TC2 ('Yellow') where "minor to moderate land damage from liquefaction is possible in future large earthquakes"; and
- TC3 ('Blue') where "moderate to significant land damage from liquefaction is possible in future significant earthquakes".

The site does not yet have a classification and it was the aim of this report to give an indication as to what category we consider the land will fall into.

Site photographs are presented in Appendix 1.

3 GEOLOGY

The site is mapped² as being underlain by brownish grey river alluvium.

4 GEOHAZARDS

4.1 Seismicity

Historically, Christchurch City has been considered to be in a region of low concentrations of active faults and seismicity. However, the Canterbury region has recently had three earthquakes with magnitude greater than 6. As a result, there is a heightened level of seismic risk stemming from the recently discovered Greendale, Lyttelton and Port Hills Faults. The recent seismic activity in the Canterbury region is currently considered to have increased the probability of another large (M6.0-6.9) earthquake to 21%³ between the time of writing and November 2012.

Preliminary mapping⁴ of the recent faulting in Canterbury illustrates the approximate locations of the Greendale Fault and sub-surface Lyttelton Fault rupture, the distribution of associated aftershocks since the 4th of September 2010 event, and known active faults in the Canterbury area. Large regional areas of faulting^{2,5} namely the Ashley Fault, Porters Pass-Amberley Fault Zone, and the Hope and Alpine Faults, are further afield but present a high seismic hazard risk to the Christchurch area due to the anticipated size of earthquakes generated. The largest of these faults is the Alpine Fault, which has a return period of 250-300 years and is expected to produce a M8 earthquake. The last rupture on the Alpine Fault is believed to have occurred in 1717⁶.

4.2 Liquefaction and Lateral Spreading

The site is located outside the "potentially liquefiable ground zone" and "lateral spread zone" identified by the Selwyn District Council⁷, and we consider the potential for liquefaction and lateral spreading to be very low owing to the nature of the subsurface materials¹ and depth to groundwater at the site.

Aerial photography⁸ taken in the days following the 22nd of February event shows no sign of any ejected sand and silt at the site and surrounding areas. We are unaware of any liquefaction or lateral spreading being observed within approximately 20 km of the site during the recent large earthquakes affecting the Canterbury region.

5 FIELD INVESTIGATIONS

5.1 Test Pits

Geoscience visited the site on the 30th of November 2011 and completed logging of materials from 12 machine excavated test pits. Our investigations found the geology to be consistent with published mapping, as summarised in Table 1.

Depth (m)	Material Description	Material Type	Density/Consistency
0.0 to 0.3	SILT with trace to minor sand, dark brown, dry.	TOPSOIL	Stiff
0.3 to 4.0	Sandy fine to coarse GRAVEL, brown, well graded, rounded, dry. Some SAND lenses (<0.2 m)	ALLUVIUM	Very Dense
>4.0	Inferred GRAVEL		Very Dense

Table 1:	Summary of	Typical	Subsurface	Conditions
				•••••••

Groundwater was not encountered in any of the test pits.

Full logs are presented in Appendix 2 and are written in accordance with the New Zealand Geotechnical Society 'Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes'⁹.

5.2 AutoScala (Penetrometer) Testing

The AutoScala is a small mechanical rig that pushes a Scala Penetrometer cone into the ground. A total of 91 tests were carried out approximately every 100 m across the site by Canterbury Geotest to a maximum depth of 1.2 m below ground level. The AutoScala tests were undertaken to assess the subsurface strength profile and to help determine if ground beneath the site meets the requirements of static "good ground", defined in NZS 3604:2011¹⁰ as follows:

"Where the number of blows per 100 mm depth of penetration below the underside of the proposed footing at each test site exceeds:

- 5 down to a depth equal to twice the width of the widest footing; and
- 3 at greater depths.

Furthermore, the definition of "good ground" also excludes organic topsoil, soft or very soft peat, soft or very soft clay and / or uncertified fill below the depth of footing at any test site. Sites prone to liquefaction also do not meet under the definition of "good ground".

"Good ground" under static conditions was typically encountered immediately beneath the topsoil layer (on average 0.3 m) and at a maximum depth of 0.45 m below ground level.

AutoScala results including a summary sheet are presented in Appendix 3.

5.3 Environment Canterbury (ECan) Borehole Logs

A review was conducted of deep ECan borehole logs in the general area. A total of four representative borelogs from holes located either on or immediately adjacent to the site were reviewed. The logs from these holes are presented in Appendix 4 and indicate the site is underlain by a mixture of claybound and sandy gravels to depths of at least 114 m below ground level. Some minor clay and peat layers are recorded below approximately 28 m depth in some of the boreholes. None of these materials are prone to liquefaction.

5.3.1 Groundwater

Groundwater is recorded on the ECan borehole logs at depths ranging from 9 to 11 m below ground level.

6 CONCLUSIONS

Based on our site investigation and assessment we conclude the following:

- Geotechnical materials underlying the site typically comprise of 0.3 m of topsoil underlain by very dense gravel to depths of at least several tens of metres and probably hundreds of metres depth.
- AutoScala testing indicates "good ground" can typically be found immediately below the topsoil layer and at a maximum depth of 0.45 m.
- ECan borehole logs in the area indicate groundwater can be found at between 9 and 11 m below ground level.
- There is no evidence of any liquefaction having occurred at or near the site during recent large earthquakes. We consider the likelihood of liquefaction or lateral spreading occurring at the site to be very low based on the depth to groundwater and the nature of the sub-surface materials (namely gravel).

7 RECOMMENDATIONS

Based on our findings we consider the site characteristics to be consistent with a Technical Category 1 (TC1) classification where:

"Future land damage from liquefaction is unlikely. You can use standard foundations for concrete slabs or timber floors. Foundation requirements changed in 2011 and information is available on the Department of Building and Housing's website"¹:

It should be noted that while we consider the site suitable for residential sub-division, this report is not intended for 224c sub-division sign off or for building consent purposes. The Selwyn District Council guidelines indicate further geotechnical testing may be required for these consents. A final report will be provided following earthworks to confirm the site technical category. We assume this will be required as a condition of subdivision consent.

8 REFERENCES

- 1 The Department of Building and Housing, October 2011: Preview of the Update to: Guidance on House Repairs and Reconstruction following the Canterbury Earthquakes (http://www.dbh.govt.nz/earthquake--reconstruction-guidance-preview)
- 2 Brown, L.J., Weeber, J.H., 1992: Sheet 1 Geology of the Christchurch Urban Area 1:25,000. Institute of Geological and Nuclear Sciences, Lower Hutt.
- 3 <u>http://www.geonet.org.nz/var/storage/images/media/images/news/2011/chch_seismicity_04_09_1</u> <u>1/58577-1-eng-GB/Chch_Seismicity_04_09_11.jpg</u>
- 4 <u>http://www.geonet.org.nz/canterbury-quakes/aftershocks/</u>
- 5 Rattenbury, M.S.; Townsend, D.B.; Johnston, M.R., 2006: Sheet 13 Geology of the Kaikoura Area 1:250,000. Institute of Geological and Nuclear Sciences, Lower Hutt.
- 6 Pettinga J.R., Yetton M.D., Van Dissen R.J., and Downes G., 2001: Earthquake Source Identification and Characterisation for the Canterbury Region, South Island, New Zealand, Bulletin of the New Zealand Society for Earthquake Engineering, Vol 34, No. 4, pp 282-317
- 7 Selwyn District Council 2011: 2010 Canterbury Earthquake Liquefaction Report, Issue 05.6; Prepared by Geotech Consulting Ltd.
- 8 http://koordinates.com/#/layer/3185-christchurch-post-earthquake-aerial-photos-24-feb-2011/

- 9 New Zealand Geotechnical Society, 2005: Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes.
- 10 Standards Association of New Zealand, 2011: Timber Framed Buildings New Zealand, NZS 3604:2011. Standards New Zealand, Wellington.

We also acknowledge the New Zealand GeoNet project and its sponsors EQC, GNS Science and LINZ, for providing data used in this report.

9 LIMITATIONS

- (i) This report has been prepared for the use of our client, R.D. Hughes Developments Ltd, their professional advisers and the relevant Territorial Authorities in relation to the specified project brief described in this report. No liability is accepted for the use of any part of the report for any other purpose or by any other person or entity.
- (ii) Assessments made in this report are based on the ground conditions indicated from published sources, site inspections and subsurface investigations described in this report based on accepted normal methods of site investigations. Variations in ground conditions may exist between test locations and therefore have not been taken into account in the report.
- (iii) This Limitation should be read in conjunction with the IPENZ/ACENZ Standard Terms of Engagement.

We trust that this information meets your current requirements. Please do not hesitate to contact the undersigned on 03 328 9012 if you require any further information.

For and on behalf of Geoscience Consulting (NZ) Ltd,

Greg Martin Associate Engineering Geologist



FIGURES

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APPENDIX 1 Site Photographs

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Photo 1: View looking north east from centre of site.

Photo 2: View looking south.

Photo 3: View looking south east.



Photo 4: View looking west.





Photo 5: View of sub surface material in Test Pit 1.



Photo 6: Gravel removed from Test Pit.

Date taken	30/11/11	Client	R. D. Hughes Developments Ltd					
Taken by	RC	Project	Foster Block, Goulds Road, Rolleston					
Approved by	GM	Description	Site Photographs					
Scale	N/A	Photo No.	1 to 6 Project Number 11283_1					



APPENDIX 2

Test Pit Logs

Geoscience Consulting (NZ) Limited 5 Norwich Quay, Lyttelton 8062, New Zealand PO Box 110, Lyttelton 8841, New Zealand T (+64) (3) 328 9012 F (+64) (3) 328 9013 www.nzgeoscience.co.nz



	6	GEOSC	ENCE				Test	Pit N	No.	7	ΓP01	
										1	of	1
Eı	ngine	ering Log -	Test P	it			Proje	ect No).	1	1283	}
Cli	ient:	R. D. Hughes	Develo	pments	Ltd		Date	Starte	ed: 3	0/1	1/201	1
Pri	incipa	I: Kelvin Back					Date	Comp	oleted: 3	0/1	1/201	1
Pre	oject:	Foster Block					Logg	ed By	/:	JC	C/RC	
Те	st Pit I	Location: Refe	er to Site	Locatio	on Plan		Chec	ked E	By:	(GM	
Ex	cavatio	Machine Type: 16 n Dimensions: 2.0 Vane No.: N/	Tonne Di 00 by A	gger 3.00								
	Excav	ation Informat	tion			Material Su	ıbstan	се				
Material	Water	Notes, samples, tests etc	Depth (m)	Graphic Log	Classification Symbol	Material Soil - soil type, colour, structure, grading, bedding, plasticity, sensitivity; Secondary and minor components Rock - colour, fabric, rock type; discontinuities; additional information	Moisture Condition	Consistency / Density Index	Shear Vane (Dial Readings kPa)	(Sc Blows/	a la 100mm) دو می
			0.2 0.4	тс тс ******	ML	SILT with minor sand; dark brown. Low plasticity. [TOPSOIL]	D	St				-
ALLUVIUM ALLUVIUM			$ \begin{array}{c} 0.6 \\ 0.8 \\ 0.8 \\ 1 \\ 1.2 \\ 1.2 \\ 1.4 \\ 1.6 \\ 1.8 \\ 1.8 \\ 2 \\ 2.2 \\ \end{array} $		GW GW	Sandy fine to coarse GRAVEL; brown, rounded, well graded. Sand is medium to coarse.	D-M	MD				

Coarse SAND lense.

GW

ŝ

EOH: 3.6 m Termination: Target depth Notes: Met target depth at 3.6 m. No groundwater encountered.

2.2 2.4

2.6 2.8

3 3.2

34 3.6





D-M

D

EOH: 3 m	
Termination: Target depth	

Notes:

ALLUVIUM

Met target depth at 3.0 m. No groundwater encountered.

1.6 1.8 2

2.2 2.4 2.6 2.8 GW



	6	GEOSC	ENCE				Test	Pit N	NO.	TP03	
							Shee	t		1 of 1	
Eı	Engineering Log - Test Pit							ct No).	11283	
Cl	ient:	R. D. Hughes	Develo	pments	Ltd		Date	Starte	ed: 3	0/11/2011	
Pr	incipa	I: Kelvin Back					Date	Comp	oleted: 3	0/11/2011	
Pr	oject:	Foster Block					Logg	ed By	/:	JC/RC	
Те	st Pit l	Location: Refe	er to Site	Locatio	n Plan		Chec	ked E	By:	GM	
Ex	cavatio	Machine Type: 16 n Dimensions: 2.0 Vane No.: N/	Tonne Di)0 by A	gger 3.00					-		
	Excav	vation Informat	tion			Material S	ubstan	се			
Material	Water	Notes, samples, tests etc	Depth (m)	Graphic Log	Classification Symbol	Material Soil - soil type, colour, structure, grading, bedding, plasticity, sensitivity; Secondary and minor components Rock - colour, fabric, rock type; discontinuities; additional information	Moisture Condition	Consistency / Density Index	Shear Vane (Dial Readings kPa)	Scala (Blows/100mm) ର ବ ହ ଛ	
- I			0.2	「 「 TS 並	ML	SILT with trace fine sand; dark brown, friable. Low plasticity. [TOPSOIL]	D	St		-	
	ł: 4 m nination: T	àrget depth	$\begin{array}{c} 0.4 \\ - 0.6 \\ - \\ 0.8 \\ - \\ 1 \\ - \\ 1.2 \\ - \\ 1.2 \\ - \\ 1.4 \\ - \\ 1.6 \\ - \\ 2.2 \\ - \\ 2.4 \\ - \\ 2.6 \\ - \\ 2.8 \\ - \\ 2.8 \\ - \\ 3.2 \\ - \\ 3.4 \\ - \\ 3.6 \\ - \\ 3.8 \\ - \\ 4 \\ - \\ 3.8 \\ - \\ 4 \\ - \\ 4 \\ - \\ 4 \\ - \\ 1 \\ - \\$		GW	Sandy fine to coarse GRAVEL; brown, well graded, rounded. Sand is fine to coarse.	D-M	MD			
Met	es: target dept	th at 4.0 m. No groundwa	ter encounte	red.							
Ske	etcn/P	noto									





Engineering Log - Test Pit

Client: R. D. Hughes Developments Ltd

Principal: Kelvin Back

Project: Foster Block

Test Pit Location: Refer to Site Location Plan

Machine Type:16 Tonne DiggerExcavation Dimensions:2.00by3.00

Test Pit No.	TP04
Sheet	1 of 1
Project No.	11283
Date Started:	30/11/2011
Date Completed:	30/11/2011
Logged By:	JC/RC
Checked By:	GM

		Vane No.: N/	A								
	Excav	vation Informat	tion			Material Sub	ostan	ce			
Material	Water	Notes, samples, tests etc	Depth (m)	Graphic Log	Classification Symbol	Material Soil - soil type, colour, structure, grading, bedding, plasticity, sensitivity; Secondary and minor components Rock - colour, fabric, rock type; discontinuities; additional information	Moisture Condition	Consistency / Density Index	Shear Vane (Dial Readings kPa)	Scala (Blows/100mm	1)
			$\begin{array}{c} 0.2 \\ 0.4 \\ 0.6 \\ 0.8 \\ 0.8 \\ 1 \\ 1.2 \\ 1.4 \\ 1.6 \\ 1.8 \\ 1$		ML GW	SILT with trace sand; dark brown, friable. Low plasticity. [TOPSOIL] Sandy fine to coarseGRAVEL; brown, well graded, rounded. Sand is fine to coarse.	D	St D			
-			2		GW	Medium SAND lense	D-M	D			-

EOH: 2 m Termination: Target depth

Notes:

Met target depth at 2.0 m. No groundwater encountered.





coarse

GW

-	_ 2	1900 - 40 1919 - 60 - 40	
	2.2		
-	2.4 -		
F	2.6		
		€ * * * *	
-	_2.0	1927 HODA 1927 HODA	
	3 -	ం *్ ^ ఫం	

EOH: 3 m Termination: Target depth

Notes:

ALLUVIUM

Met target depth at 3.0 m. No groundwater encountered.

Sketch/Photo





EOH: 3 m Termination: Target depth

ALLUVIUM

Notes: Met target depth at 3.0 m. No groundwater encountered.

.6

8

22

2.6 2.8 GW





Engineering Log - Test Pit

Client: R. D. Hughes Developments Ltd

Principal: Kelvin Back

Project: Foster Block

Test Pit Location: Refer to Site Location Plan

Machine Type: 16 Tonne Digger Excavation Dimensions: 2.00 by 3.00 Test Pit No.TP07Sheet1 of 1Project No.11283Date Started:30/11/2011Date Completed:30/11/2011Logged By:JC/RCChecked By:GM

	cavatio	Vane No.: N/	A by	0.00							
	Excav	ation Informat	tion		Material Substance						
Material	Water	Notes, samples, tests etc	Depth (m)	Graphic Log	lassification Symbol	Material Soil - soil type, colour, structure, grading, bedding, plasticity, sensitivity; Secondary and minor components Rock - colour, fabric, rock type; discontinuities; additional information	Moisture Condition	Consistency / Density Index	Shear Vane (Dial Readings kPa)	Scala (Blows/100mm)	
			0.2 0.4 0.6 0.8 1 1.2 1.4		ML	SILT with minor sand; dark brown. Low plasticity. [TOPSOIL] Sandy fine to coarse GRAVEL; brown, rounded, well graded. Sand is medium to coarse.	D	St MD			
- - - EOł	1: 2 m		1.6 1.8 2							-	

Termination: Target depth Notes:

Met target depth at 2.0 m. No groundwater encountered.









Engineering Log - Test Pit

Client: R. D. Hughes Developments Ltd

Principal: Kelvin Back

Project: Foster Block

Test Pit Location: Refer to Site Location Plan

Machine Type: 16 Tonne Digger Excavation Dimensions: 2.00 by 3.00 Test Pit No.TP09Sheet1 of 1Project No.11283Date Started:30/11/2011Date Completed:30/11/2011Logged By:JC/RCChecked By:GM

		Vane No.: N/	A								
	Excav	ation Information	tion	Material Substance							
Material	Water	Notes, samples, tests etc	Depth (m)	Graphic Log	Classification Symbol	Material Soil - soil type, colour, structure, grading, bedding, plasticity, sensitivity; Secondary and minor components Rock - colour, fabric, rock type; discontinuities; additional information	Moisture Condition	Consistency / Density Index	Shear Vane (Dial Readings kPa)	Scala (Blows/100mm)	
			0.2 0.4 0.6 0.8 1 1.2 1.4 1.6 1.8		ML GW	Sllt with trace sand; dark brown. Low plasticity. [TOPSOIL] Sandy fine to coarse GRAVEL; brown, well graded, rounded. Sand is fine to coarse.	D	St D			
F)H ·2m		2	· · · · · ·							

Termination: Target depth

Notes: Met target depth at 2.0 m. No groundwater encountered.





Sandy fine to coarse GRAVEL; brown,

well graded, rounded. Sand is fine to

medium.

Medium SAND lense.

L-MD

D

EOH: 2 m Termination: Target depth

Notes:

ALLUVIUM

Met target depth at 2.0 m. No groundwater encountered.

0.4

0.6 08

2

1.6

18

2

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GW

GW





			-					l `		
	-	0.2 0.4 0.6	ML GW	SILT with trace sand; dark brown. Low plasticity. [TOPSOIL] Sandy fine to coarse GRAVEL; brown, rounded, well graded. Sand is fine to medium.	<i>_</i> /	D	St MD			
ALLUVIUM			GW	Medium SAND lense. Medium SAND lense.	_		L			
		$\begin{array}{c} 1.6 \\ 1.8 \\ 2 \\ 2 \\ 2.2 \\ 2.2 \\ 2.4 \\ 2.4 \\ \end{array}$	GW							

EOH: 2.5 m

Termination: Target depth Notes:

Met target depth at 2.5 m. No groundwater encountered.





		Vane No.: N/	A	_							
	Excav	vation Information	tion	Material Substance							
Material	Water	Notes, samples, tests etc	Depth (m)	Graphic Log	Classification Symbol	Material Soil - soil type, colour, structure, grading, bedding, plasticity, sensitivity; Secondary and minor components Rock - colour, fabric, rock type; discontinuities; additional information	Moisture Condition	Consistency / Density Index	Shear Vane (Dial Readings kPa)	Scala (Blows/100mm) ম ৰ ৩ ৩	
			0.2 0.4 0.6 0.8 1 1.2 1.4 1.6 1.6 1.8 2		ML SW GW	SILT with trace sand; dark brown. Low plasticity. [TOPSOIL] Fine to medium SAND; light brown. Well graded. Sandy fine to coarse GRAVEL; brown, well graded, rounded. Sand is fine to coarse.	D	St MD D			
EO	H:2m										

Termination: Target depth

Notes: Met target depth at 2.0 m. No groundwater encountered.





APPENDIX 3

AutoScala (Penetrometer) Logs









Foster Block Site 3 Canterbury Geotest AutoScala Machine



Foster Block Site 4 Canterbury Geotest AutoScala Machine







Foster Block Site 10 Canterbury Geotest AutoScala Machine 1/12/2011





Foster Block Site 14 Canterbury Geotest AutoScala Machine



Foster Block Site 15 Canterbury Geotest AutoScala Machine



Foster Block Site 16 Canterbury Geotest AutoScala Machine



























Equivalent Scala blows/100mm





Foster Block Site 56 Canterbury Geotest AutoScala Machine





Foster Block Site 58 Canterbury Geotest AutoScala Machine



Foster Block Site 59 Canterbury Geotest AutoScala Machine



Foster Block Site 60 Canterbury Geotest AutoScala Machine





Foster Block Site 62 Canterbury Geotest AutoScala Machine



Foster Block Site 64 Canterbury Geotest AutoScala Machine

















Foster Block Site 83 Canterbury Geotest AutoScala Machine



Foster Block Site 84 Canterbury Geotest AutoScala Machine







Foster Block Site 90 Canterbury Geotest AutoScala Machine



Foster Block Site 91 Canterbury Geotest AutoScala Machine





APPENDIX 4

ECan Borehole Logs

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Borelog for well M36/0204

-5

-10

-15_

-20

-25

Gridref: M36:614-326 Accuracy : 4 (1=best, 4=worst) Ground Level Altitude : 39.42 +MSD Driller : J W Horne (& Co) Drill Method : Unknown Drill Depth : -27.4m Drill Date : 1/04/1975

00=000

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00000

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- 25.6m

- 26.2m

- 27.4m



	ri
Big stones	br?
Loose claywash gravel	
	br?

Environment Canterbury



Borelog for well M36/7204

Gridref: M36:6177-3216 Accuracy : 4 (1=best, 4=worst) Ground Level Altitude : 36 +MSD Driller : Smiths Welldrilling Drill Method : 1st Rotary 2nd Cable Drill Depth : -114m Drill Date : 23/08/2000



Environment Canterburv

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